

Verification Dovetail-Connection

according to building approval Z-9.1-649 (validity 18 June 2018 - 5 October 2022)

Connection & Geometry

double-sided connection

material: laminated timber
solidity: GL24c according to DIN EN 14080:2013-09

width secondary beam b_N : 120 mm
height secondary beam h_N : 200 mm

width main beam b_H : 160 mm
height main beam h_H : 280 mm

connection angle φ : 45°

milling angle β : 10°

pin cone angle γ : 4°

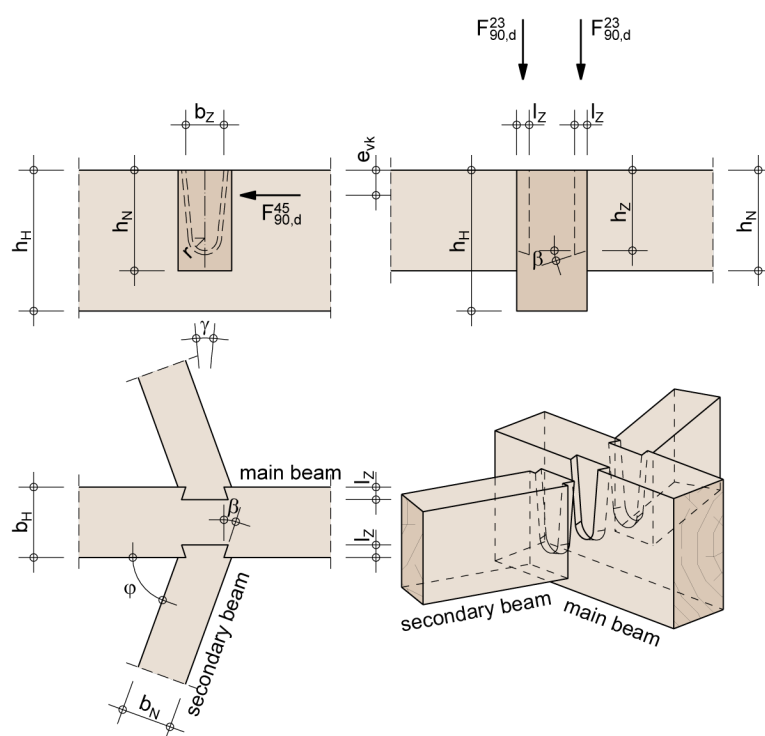
length pin l_Z : 25 mm

width pin b_Z : 110 mm

height pin h_Z : 193 mm

pin hole radius r : 50 mm

dimensional tolerances: +/- 0.2 mm



Loads

Load $F_{23,90,d}$: 13.00 kN

Load $F_{45,90,d}$: 0.00 kN

e_{vk} : 0 mm

Service class: NKL1 - heated interiors

KLED: medium

modification factor k_{mod} : 0.8

Verification:

$$1.00 \leq 1.00$$

Verification fulfilled

Construction tip

Building approval Z-9.1-649 must be observed.

Stress resistance in insertion direction

$$k_n: \quad 6.50$$

$$\alpha = \cos(\delta) * \frac{h_Z - r}{h_N} = \cos(0) * \frac{193 - 50}{200} = 0.71$$

$$k_v = \min \left\{ \begin{array}{l} 1 \\ \frac{k_n}{\sqrt{h_N} * (\sqrt{\alpha} * (1 - \alpha) + 0.4 * \frac{l_Z}{h_N} * \sqrt{\frac{1}{\alpha} - \alpha^2})} \end{array} \right.$$

$$= \min \left\{ \begin{array}{l} 1 \\ \frac{6.50}{\sqrt{200} * (\sqrt{0.71} * (1 - 0.71) + 0.4 * \frac{25}{200} * \sqrt{\frac{1}{0.71} - 0.71^2})} \end{array} \right.$$

$$= \min \left\{ \begin{array}{l} 1 \\ 0.92 \end{array} \right.$$

$$k_{ab}: \quad 0.80$$

$$f_{t,90,d}: \quad 0.308 \text{ N/mm}^2 \quad (\text{with } f_{t,90,k} = 0.5 \text{ N/mm}^2 \text{ according to approval})$$

$$f_{v,d}: \quad 1.538 \text{ N/mm}^2 \quad (\text{with } f_{v,k} = 2.5 \text{ N/mm}^2 \text{ according to approval})$$

$$F_{90,Rd}^{23} = \min \left\{ \begin{array}{l} k_{ab} * \frac{h_Z}{h_Z - r} * \left(6.5 + \frac{18 * (h_H - h_Z + r)^2}{h_H^2} \right) * (t_{ef} * h_H)^{0.8} * f_{t,90,d} \\ \frac{k_v * b_N * (h_Z - r)}{1.5} * f_{v,d} \end{array} \right.$$

$$= \min \left\{ \begin{array}{l} 0.80 * \frac{193}{193 - 50} * \left(6.5 + \frac{18 * (280 - 193 + 50)^2}{280^2} \right) * (100 * 280)^{0.8} * 0.308 \\ \frac{0.92 * 120 * (193 - 50)}{1.5} * 1.538 \end{array} \right.$$

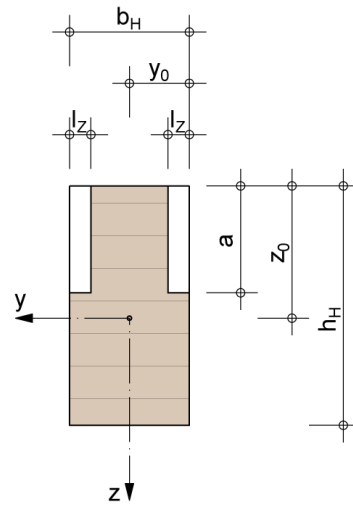
$$= \min \left\{ \begin{array}{l} 12.97 \\ 16.14 \end{array} \right.$$

$$\text{stress resistance } F_{90,Rd}^{23}: \quad 12.97 \text{ kN}$$

Verification in insertion direction:	$\frac{F_{90,d}^{23}}{F_{90,Rd}^{23}} = \frac{13.00}{12.97} =$	$1.00 \leq 1.00$
--------------------------------------	--	------------------

Cross-sectional weakening

width main beam b_H :	160 mm
height main beam h_H :	280 mm
height a :	197.4 mm
length pin l_Z :	25 mm
position of centre y_0 :	80.0 mm
position of centre z_0 :	151.7 mm
moment of inertia I_y :	23905 cm ⁴



moment of inertia of the unattenuated main beam:

$$I_{y,H} = \frac{b_H * h_H^3}{12} = \frac{160 * 280^3}{12} * 10^{-4} = 29269 \text{ cm}^4$$

required cross-sectional height with the same moment of inertia I_y related to the unattenuated cross-sectional of the main beam:

width beam b_H :	160 mm
height beam h_{erf} :	300 mm
height a :	197.4 mm
length pin l_Z :	25 mm
position of centre y_0 :	80.0 mm
position of centre z_0 :	163.3 mm
moment of inertia I_y :	29525 cm ⁴

